

**BMP Facility Design Calculations**

Plan Name: METRO PLACE- PARCEL C-1 Date: Feb. 29, 2000  
 Plan Number: 5169/AD/E Engineer: CEH/MP

**Water Quality Narrative**  
 The proposed 2.65 acre residential development which includes parcel C-1, requires a stormwater management plan that will be designed to meet the requirements of the Fairfax County Public Facilities Manual, Chapter 11.2. The facility will be privately owned and maintained.

**II. Watershed Information**

Part 1: List all of the Subareas and 'C' Factors used in the BMP Computations

Subarea Designation and Description	'C'	Acres
A1 ON-SITE TREATED (1)	0.84	1.65
A2 OFF-SITE TREATED	0.90	0.21
A3 ON-SITE NOT TREATED	0.83	1.00

NOTE: Rational formula 'C' factors are taken from the general zoning values listed in Appendix 4-1 or 4-2 depending on the location of the BMP facility (Fairfax County Public Facilities Manual Chart A6-19 or Prince William County Design and Construction Standards Manual Exhibit 1).

Part 2: Compute the Weighted Average 'C' Factor for the Site

(A) Area of the site	(a) 2.65	acres
(B) Subarea Designation	'C'	Acres
(1)	(2)	(3)
A1 ON-SITE TREATED	X	X
A2 OFF-SITE NOT TREATED	0.84	1.65
A3 ON-SITE NOT TREATED	0.63	1.00
	X	X
	X	X
	X	X
	X	X
	X	X
	X	X
	X	X
(C) Weighted average 'C' factor	(b) Total = 2.02	
	(b) / (a) = (c) 0.76	

Part 3: Compute the Total Phosphorus Removal for the Site

Subarea Designation	BMP Type	Removal Eff. (%)	Area	'C' Factor Ratio	Product
A1	SAND FILTER	60	1.65	0.76	41.29
A2	SAND FILTER	60	0.21	0.76	5.63
		X	X	X	X
		X	X	X	X
		X	X	X	X
		X	X	X	X
		X	X	X	X
		X	X	X	X
		X	X	X	X
		X	X	X	X
		X	X	X	X
(a) Total =					46.92 %

Part 4: Determine Compliance with Phosphorus Removal Requirement

(A) Select Requirement (a) 40 %

- Water Supply Overlay District = 50% (Fairfax County and Occoquan Watershed) = Prince William County
- Chesapeake Bay Preservation Area (New Development) = 40% (Fairfax County)
- Chesapeake Bay Preservation Area (Predevelopment) = 50% (Prince William County)

(B) If Line 3(a) 48.26 > Line 4(a) 40 then Phosphorus removal requirement is satisfied.

WORKSHEET 1: CONCENTRATIONS COMMON TO ALL SAND FILTER BAYS  
 SAND FILTER BMP COMPUTATIONS

Part 1: Compute the Design Stormwater Runoff Volume (V <sub>R</sub> )	Part 2: Compute Filter Volume (V <sub>F</sub> )
structures = 56,375 ft <sup>2</sup>	
parking lot = 1,100 ft <sup>2</sup>	
roadway = 1,400 ft <sup>2</sup>	
other = 1,800 ft <sup>2</sup>	
total = 60,675 ft <sup>2</sup>	
	V <sub>R</sub> = 1816 ft <sup>3</sup>
	V <sub>F</sub> = 1816 ft <sup>3</sup>
	V <sub>AV</sub> = 1816 ft <sup>3</sup>
	V <sub>AD</sub> = 1816 ft <sup>3</sup>

Part 3: Identify critical site parameters:  
 storm sewer invert at proposed connection point = ft  
 length of outflow line (BMP - storm sewer) = ft  
 minimum BMP outflow invert or minimum 0.5% grade = ft  
 site plan surface elevation at BMP location = ft  
 inflow invert to BMP from drainage system plan = ft  
 flow splitter weir or bypass pipe invert (usually set at maximum BMP ponding depth) = ft  
 BMP outflow possible by gravity = ft

Part 4: Consideration for Detention Sand Filter:  
 Part 4.1: Considering data on Worksheet 1, select maximum ponding depth over filter:  
 2h = 7.82 ft;  
 h = 3.91 ft

Part 5: Compute Minimum Area of Filter (A<sub>F</sub>) and Sediment Pool (A<sub>S</sub>):  
 a) If 2h > 2.67 ft, use the formula:  
 A<sub>F</sub> = A<sub>AV</sub> = 5451.4 (d<sub>50</sub> + h)  
 = 1945 x 2.08 x 1.5 = 3143.1 ft<sup>2</sup>

b) If 2h < 2.67 ft, use the formula:  
 A<sub>F</sub> = A<sub>AV</sub> = 1816 / ((4.1h + 0.9) / (4.1h + 0.9))  
 = 1816 / ((4.1 x 3.91 + 0.9) / (4.1 x 3.91 + 0.9)) = 567 ft<sup>2</sup>

Part 6: Consider Site Constraints. Select filter width (W<sub>F</sub>) and sediment pool width (W<sub>S</sub>) and compute filter length (L<sub>F</sub>) and sediment filter area (A<sub>S</sub>):  
 W<sub>F</sub> = W<sub>S</sub> = 5.67 ft;  
 L<sub>F</sub> = L<sub>S</sub> = A<sub>F</sub> / W<sub>F</sub> = 3143.1 / 5.67 = 554.3 ft

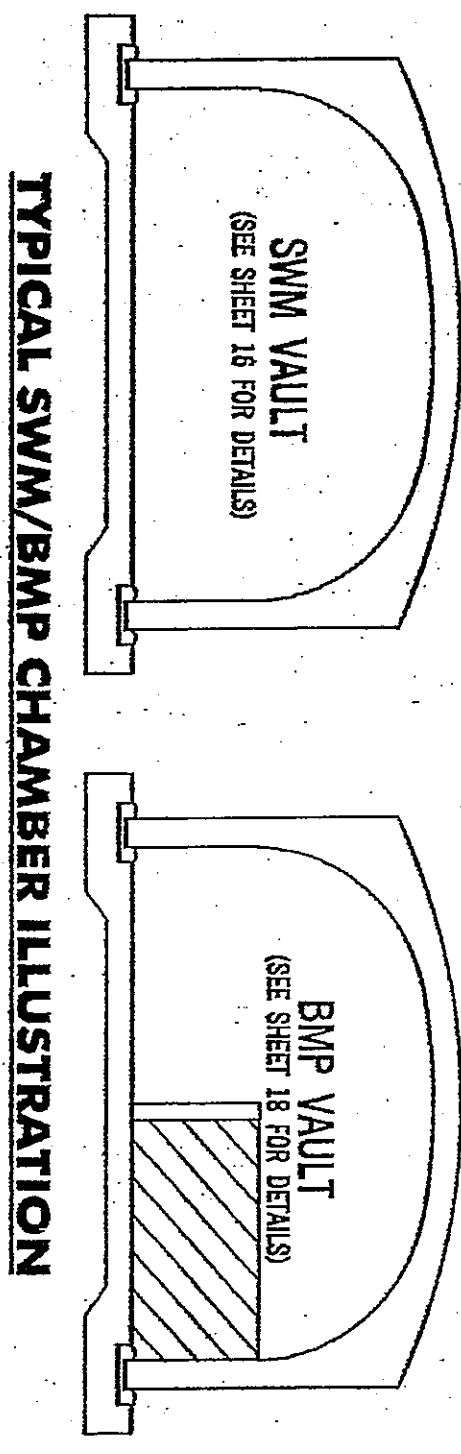
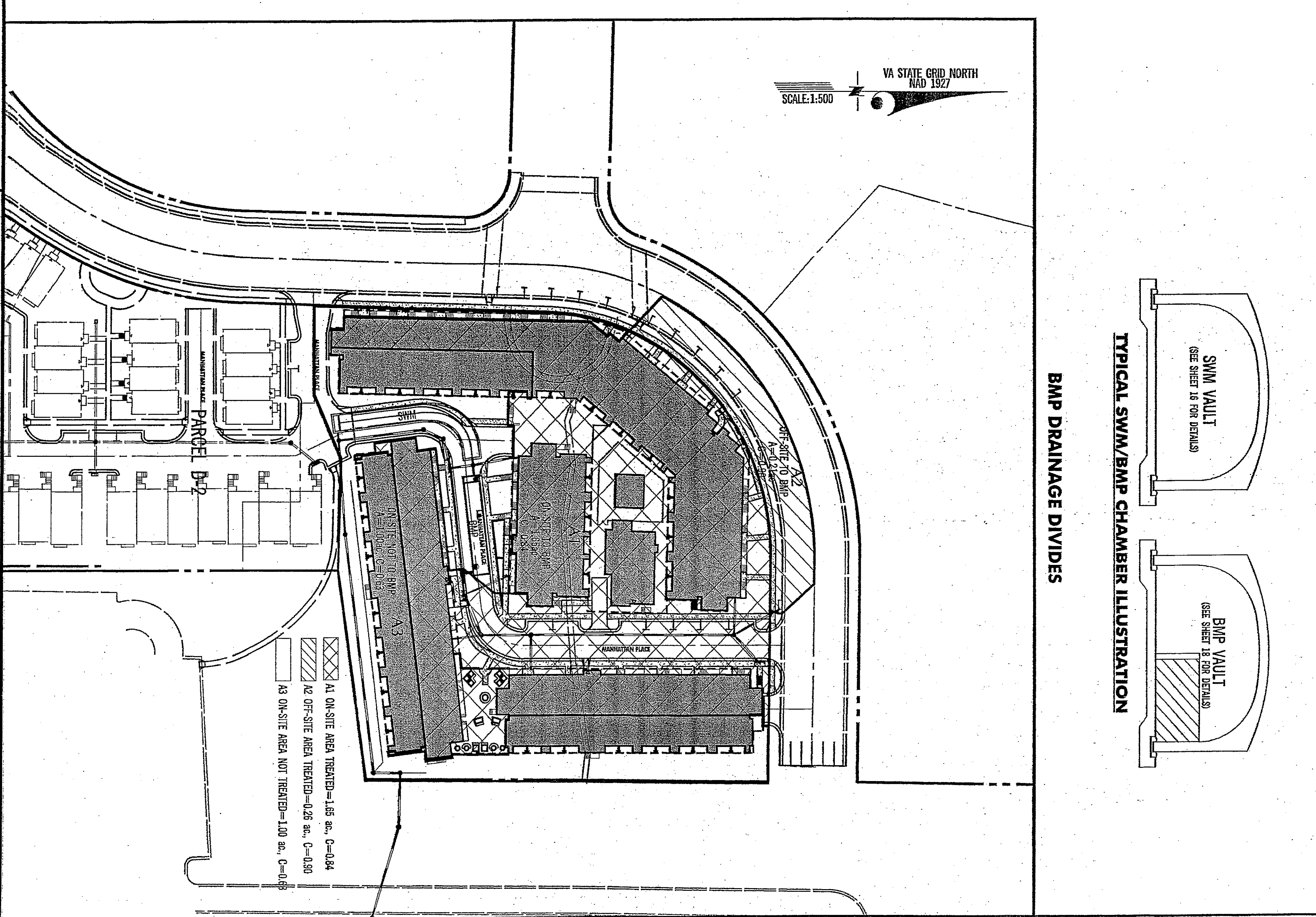
Part 7: Compute Storage in Filter Voids (V<sub>V</sub>):  
 V<sub>V</sub> = A<sub>F</sub> x d<sub>F</sub> x 0.4 = 395.94 x 1.5 x 0.4 = 237.56 ft<sup>3</sup>

Part 8: Compute flow through filter during filling period (Q<sub>F</sub>):  
 Q<sub>F</sub> = K<sub>F</sub> / (d<sub>F</sub> + h<sub>F</sub>) ; use K = 2 ft/day = 0.0833 ft/hr  
 = 10.0833 x 395.94 x (1.5 + 3.91) / 1.5 = 119.00 ft<sup>3</sup>

Part 9: Compute net volume to be stored awaiting filtration (V<sub>AV</sub>):  
 V<sub>AV</sub> = V<sub>R</sub> - V<sub>V</sub> - V<sub>F</sub> = 3777 - 237.56 - 119.00 = 3420 ft<sup>3</sup>

Part 10: Compute Storage Above Filter and Sediment Pool (V<sub>AV</sub>):  
 V<sub>AV</sub> = 2h(A<sub>F</sub> + A<sub>S</sub>) = 7.82 (395.94 + 395.94) = 6193 ft<sup>3</sup>

Part 11: Compute Storage Deficit (V<sub>D</sub>):  
 V<sub>D</sub> = V<sub>AV</sub> - V<sub>AV</sub> = 3420 - 6193 = -2773 ft<sup>3</sup>  
 IF V<sub>D</sub> < 0, SET TO PART 13. IF V<sub>D</sub> > 0, ADVISE DESIGN TO PROVIDE ADDITIONAL STORAGE.

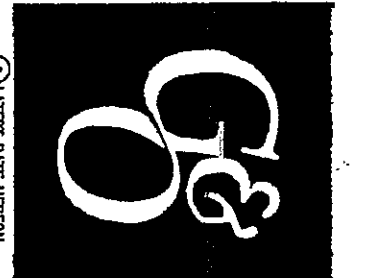
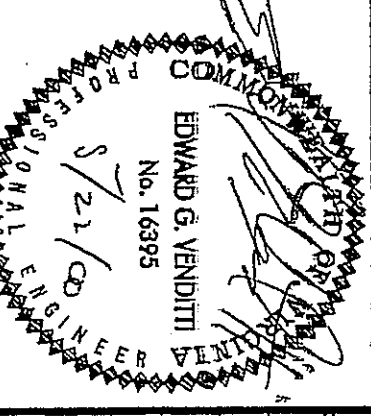


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