UTILITY TRENCHES AND MANHOLES

The excavations for the utility trenches shall be made with 1H:1V or flatter side slopes in accordance with applicable OSHA excavation standards detailed in 29 CFR, Part 1926 and shall be adequately protected against sudden cave-in or sloughing by using steel trench

The backfill in the utility trenches shall conform to the requirements of the Fairfax County, Department of Environmental Management Memorandum dated January 23, 1990, in addition to the requirements for 'Site Preparation and Earthwork'.

The backfill against the manhole structures shall conform to the requirements stated under 'Site Preparation and Earthwork'. The fill material shall not have rock fragments larger than 150mm (3 inches) and each lift shall be compacted as specified.

UNDERDRAINAGE FOR SIDEWALKS

The provision of underdrains below the sidewalk shall be governed by the current Fairfax County Public Facility Manual. Tests for plasticity index and grain-size distribution shall be conducted on representative subgrade samples to establish the need for underdrainage along the sidewalks, in sections where the gradient is 3 percent or steeper.

Moderately to highly plastic soils, (LL>40 and PI>20), if encountered at or below the subgrade elevation of the streets and roadways, shall be removed to a minimum of 600 mm (2 feet) below the street subgrade(s) and replaced with approved fill material under controlled conditions.

The soil subgrade in the paved areas, including sidewalks, curb and gutter, and driveway aprons, shall be compacted to at least ninety-five percent (95%) of the maximum dry density as determined by VTM-1 test method up to 150 mm (6 inches) below the planned subgrade elevations for controlled fills. The upper 150 mm (6 inches) of the subgrade for natural soils as well as controlled fills shall be compacted to one hundred percent (100%) of these values. The moisture content of the subgrade soils shall be within plus or minus two (±2) percentage points of the optimum moisture content.

The subgrade shall be t with VDOT requiremen basis of these data. requirements. se data. tested for laboratory C.B.R. under soaked conditions in accordance nts. The design of the pavement elements shall then be made on the

Underdrains may have to be installed behind the curb and gutters if perched conditions are encountered along their alignment. This will be decided after the grading is completed. water mass-

DEMOLITION OF OLD STRUCTURES

Any existing structures, including footings, slabs, basement walls, driveways, drainfields and septic tanks, etc., shall be removed from all building pads and pavement areas, including at least 5 feet off-sets from any building or pavement. All soils undercut below the planned grades shall be replaced with engineered fill. It is our experience that debris-laden fills are usually encountered in the vicinity of existing structures. All existing fills shall be removed to suitable-bearing native subgrade and replaced with approved structural fill.

Any water well(s) shall be abandoned and sealed as per the Fairfax County and State Health Department regulations.

Any demolition of existing building(s) and other structures shall be carried out under supervision of the Geotechnical Engineer of Record. the

RESPONSIBILITY OF DEVELOPER

According to Fairfax County Codes, review and approval of plans, specifications, and reports by the County with or without recommendations by the Geotechnical Review Board, shall in no way relieve the developer of the responsibility for the design, construction and performance of the structures and pavement on the project and damage to surrounding

excavations guidelines. I they are In Federal Register, Volume 54, No. 209 (October 1989), the United States Department of Labor, Occupational Safety and Health Administration (OSHA) amended its "Construction Standards for Excavations, 29 CFR, Part 1926, Subpart P". This document was issued to better allow for the safety of workers entering trenches or excavations. It is mandated by this federal regulation that excavations, whether they be utility trenches, basement excavations or footing excavations, be constructed in accordance with the new OSHA guidelines. It is our understanding that these regulations are being strictly enforced and if they are not closely followed, the owner and the Contractor could be liable for substantial

The Contractor is solely responsible for designing and constructing stable, temporary excavations and shall shore, slope, or bench the sides of the excavations as required to maintain stability of both the excavation sides and bottom. The Contractor's "responsible person", as defined in 29 CFR Part 1926, shall evaluate the soil exposed in the excavations as part of the Contractor's safety procedures. In no case shall slope height, slope inclination, or excavation depth, including utility trench excavation depth, exceed those specified in all local, state, and federal safety regulations.

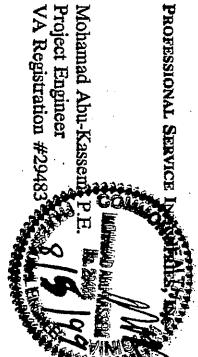
We are providing this information solely as a service to our responsibility for construction site safety or the Contractor's with local, state, and federal safety or other regulations. PSI does ler parties not assume compliance

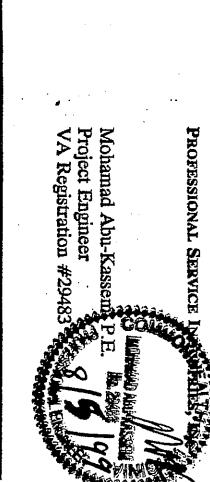
CONSTRUCTION OBSERVATIONS

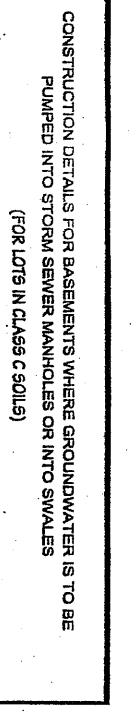
Fairfax County Public Facilities Manual uires that:

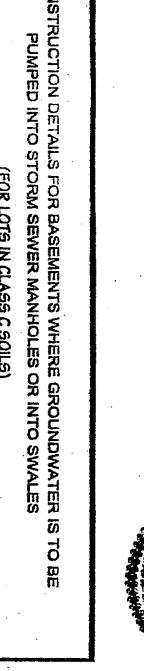
- involving nvolving problem soil shall be performed under Geotechnical Engineer of Record. the full-time
- The Geotechnical Engineer of Recoras to whether or not the work has be plans and his recommendations for varior to the issuance of residential was a superior to the instance of residential was a superior to the issuance of residential was a superior to the instance of residential was a superior to the instance of the superior to the superio

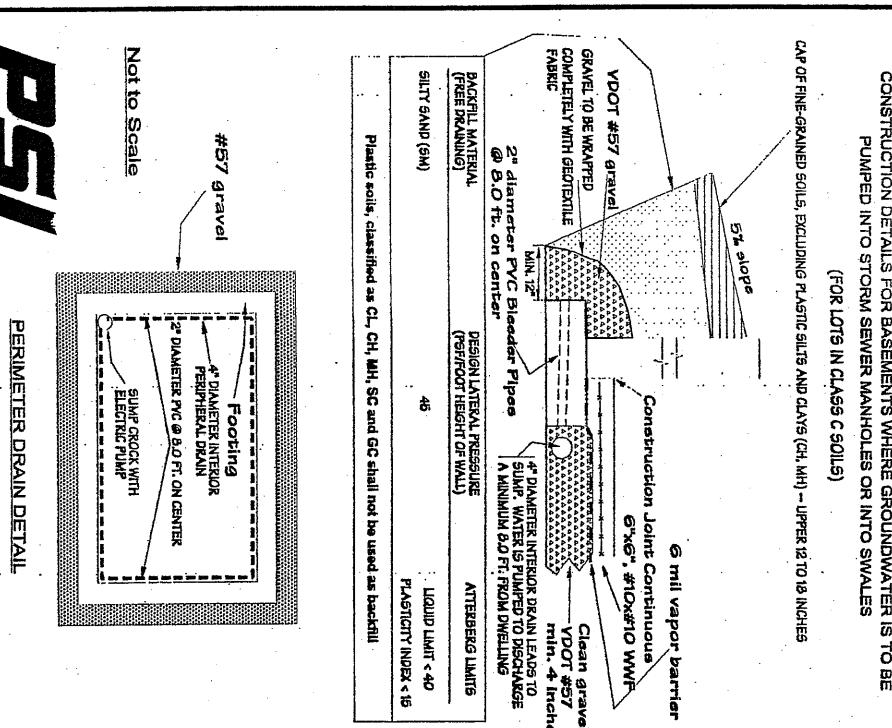
Professional Service Industries, Inc. (PSI) I have been prepared in accordance with the report dated June 17, 1999. nd certify that they in the geotechnical











SUMMARY OF LABORATORY TEST RESULTS METRO PLACE AT DUNN LORING, PARCELS C, D2, AND PSI PROJECT NO.: 463-95057

CONSTRUCTION DETAILS FOR BASEMENTS WHERE GROUNDWATER IS PUMPED INTO STORM SEWER MANHOLES OR INTO SWALES

(FOR LOTS IN CLASS A AND/OR B SOILS)

cluding plastic silts and clays (ch. MH) --

· UPPER 12 TO 18 INCHES

TERNATIVELY, THE WALLS SHOULD BE LATED WITH DECO-20 OR EQUIVALENT

oint Continuous

TEST LOCATION	TB-4	ì	TB-2
SAMPLE INTERVAL (FEET)	2.5-4		2.5-4
USCS CLASSIFICATION	. GC		ML
AASHTO CLASSIFICATION	A-7-5(8)		A-7-5(16)
NATURAL MOISTURE CONTENT (%)	18.2	20.8	- 24.9
PERCENT PASSING #200 SIEVE (%)	44.3		99.4
LIQUID LIMIT (%)	58	36	44
PLASTIC LIMIT (%)	30	25	32
PLASTICITY INDEX	28	11	12
MAXIMUM DRY DENSITY PER VTM-1 (PCF)	ang-en	***************************************	N was
OPTIMUM MOISTURE CONTENT (%)	Blance of the state of the stat		***
	•		

GRAVEL TO BE WRAPPED
COMPLETELY WITH GEOTEXTILE
FABRIC

SILTY SAND (SM)

Plastic soils, classified as CL, CH, WH, SC and GC shall not be used as backfill

2" DIAMETER PYC @ 8.0 FT. ON CENTER

SUMP CROCK WITH

Footing
4" DIAMETER INTERIOR
PERIPHERAL DRAIN

PERIMETER DRAIN DETAIL

FREE DRAINING)

2" diameter PVC Bla @ 8.0 ft. on center

edor Pipaa

4" DIAMETER INTERIOR DRAIN LEADS TO SUMP. WATER IS PUMPED TO DISCHARGE A MINIMUM 8.0 FT. FROM DWELLING

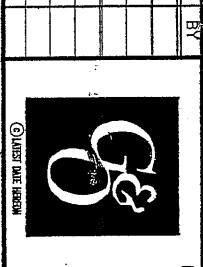
DESIGN LATERAL PRESSURE (PSF/FOOT HEIGHT OF WALL)

ATTEXBERG LIMITS

HOUD LIMIT < 40

PSI PROJECT NO.: 463-95057	METRO PLACE AT DUNN LORING, PARCELS C, D2, AND E	SUMMARY OF LABORATORY TEST RESULTS (PREVIOUS STUDY)	
•		T	

		STRATUMI				STRA	STRATUM III	
T LOCATION	B-2	B-10	B-18	B-19	B-6	B-14	B-16	B-21
MPLE INTERVAL (FEET)	0-8	0-1.5	2.5-4	2.5-4	5-6.5	0-8	13.5-15	· 0-12
S CLASSIFICATION	ML	CL	ML.	SC	ML	ML	ML	ML
SHTO CLASSIFICATION	A-4(0)	A-7-6(21.5)	A-7-6(11.4)	A-7-6(5.8)	A-4(0)	A-4(0)	A-4(0)	A-4(0)
URAL MOISTURE CONTENT (%)	18.9	24.3	17.7	15.9	21.1	18.5	35.8	23.7
RCENT PASSING #200 SIEVE (%)	58.1	82.4	61.5	47.8	93.5	82.0	94.4	71.5
JID LIMIT (%)	NP	46	49	44	NP	NP	NP	NP
STIC LIMIT (%)	ΝP	20	28	25	NP	NP	NP	NP
STICITY INDEX	NP	26	21	19	NP	NP	ЧN	NP
KIMUM DRY DENSITY PER VTM-1 (PCF)	109.1	anne				96.5	near the second	104.1
"IMI IM MOISTI IRE CONTENT (%)	185					6 66		306



Greenhorne horne & O'Mara, 22030 Inc

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GEOTECHNICAL REQUIREMENTS - PARCELS LACE AT DUNN LO C1, D2 & E

8/99 ATE 5169JADE PROJ No. 24 PP-805

 $PARCEL\ CI$ PROVIDENCE DISTRICT FAIRFAX COUNTY, VIRGINIA AS SHOWN